

Non Metallic Material Sand, Gravel And Stone On Ongkag River: Method of Mining

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Non Metallic Material Sand, Gravel And Stone On Ongkag River: Method of Mining

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Abstract - Mining of non-metallic sand, gravel, and river stones in a river channel has previously caused various damage both to the river channel itself and to the surrounding environment. Technical studies are needed to minimize the damage that can occur in mining in the river channel. Ongkag Dumoga river channel is planned to be the location of mining non-metal material for sand, gravel and river stone.

The research method was carried out by geodesic mapping to determine the profile of the river cross section, hydrological analysis with a return period of 25, 50, 100 years and hydraulic analysis on the river flow along the 600 m. The design of the post-mining river technical profile is carried out to improve the capacity of the existing river cross section. Changes in river profile are the potential volume of non-metallic material in the form of sand, gravel and river stones that become river sediment. The results of the analysis of flood discharge HSS-Snyder method get a discharge of 1249.1692 m³ / s at the 100-year return, 1178.8064 m³ / s at the 50-year return period, 1104.2372 m³ / s at the 25-year return period. The simulated flood discharge shows that there was a flood overflow in the existing river during the 25, 50 and 100 year floods. The new river technical cross section is planned to use the economic cross section of the trapezium cross section. Changes in river profiles are calculated to get a potential excavation volume of 88,347.42 m³.

Keywords - Nonmetallic material mining, plan flood discharge, economic latitude view, dumoga cost.

1. Introduction

Before 2014 the mining of nonmetallic material in the river channel or commonly called Galian C mining permit was granted by the Level II Regional Government, namely the Regent or Mayor without going through studies on the impacts that could occur, therefore there was heavy damage to the rivers especially occurred on the river Dumoga Ongkag whose material was mined before 2014.

Based on the damages that occurred, the central government issued a moratorium through Law no. 23 of 2014 which explains that non-metal mining or Galian C in rivers, permits are granted by the Level I Regional Government, namely the Governor, but first there must be a technical study of matters relating to water flow, transport sediment, and river morphology.

This study is expected to be able to control the damage that might occur in the Ongkag Dumoga river, which is one of the locations for mining sand, gravel and river stone material to a predetermined limit and can find out the material potential contained in the Dumoga Ongkag river.

2. Area Of Research

Administratively the research location is located in

Totabuan village, Lolak sub-district, Bolaang Mongondow district and geographically it is located at 0 ° 46 '48.33683 "LU and 124 ° 6" 25.0785 "East.

This study aims to meet the standards of the technical rules of river channel mining, river border management, cross-sectional capacity, river discharge in accordance with the mining laws of non-metal material, sand, gravel and river stone which can then be made a reference for miners.

3. Method of Data Collection

3.1. Secondary Data

This data can be obtained from agencies and parties deemed related to the Dumoga river, also obtained from literature, reports or notes from parties related to research. The data include:

- Watershed Map, Land Use Map, and Map of Station. Station for Ongkag Dumoga Watershed
- Rainfall data obtained from the Sulawesi River Region I (BWSS I)
- Results of previous topographic mapping.

3.2. Primary Data



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The primary data collection is carried out by field surveying in order to acquire data accurately. The identification and listing are the initial process of primary data collection to get the big picture the research location. Visual observation and

interview with the community location survey to obtain data on Dumoga river conditions in Dumoga through interviews with communities around the river and the local government.



Fig 1: Map of Ongkag River

4. Method Of Data Analysis

Analysis of the data in question is to do an analysis of Frequency and Distribution of hourly rain from rainfall data that has been obtained previously based on the Probability Distribution and proceed with calculating the intensity of rainfall. Flood discharge analysis was carried out using various methods, the results of the flood discharge based on the recycle will be carried out hydraulic analysis using the HEC-RAS software so that the flood water profile can be identified, then the hydraulic profile of the excavated river profile is calculated to obtain the volume excavation plan.

4.1. Rainfall Data

Rainfall data is obtained from the maximum daily rainfall data of the 4 closest Rain Observation Stations, namely the Toraut Rain, Konarom, Matayangan and Pusian Observation Stations. The data was obtained from BWSS I (Balai Sungai Sulawesi I) of North Sulawesi Province. The observation was carried out for 10 years (2008 - 2017).

4.2 Data Quality Analysis

Outlier data test is performed before analyzing rainfall data with the aim to find out whether there is extreme rainfall data due to negligence in recording or extreme conditions occur. This outlier data test is performed for high outlier data and low outlier data with the test requirements based on the skewness coefficient (CsLog). Outlier test results get no outlier data in every rainfall data available.

4.3. Thiessen Polygon Method Average Rainfall Analysis

Average Rainfall Analysis is performed to get the average

rainfall value by calculating the area of influence based on the Thiessen Polygon.

4.4. Rainfall Probability Data Analysis

Determination of the type of distribution in accordance with the data is done by testing the rainfall data using statistical parameters. This analysis is calculated using rainfall data that has been corrected from the results of the outlier data analysis. Some probability distribution methods that can be used are: Gumbel, Normal, Normal Log, and Type Person Log III. In the Calculation of statistical parameters for the observation data are as follows:

Central tendency

$$\bar{X} = \frac{\sum xi}{n} = \frac{956,90}{10} = 95,69 \text{ mm}$$

$$\log \bar{X} = \frac{\sum \log xi}{n} = \frac{19,76}{10} = 1,975 \text{ mm}$$

Standard deviation source data

$$S = \sqrt{\frac{1}{n-1} \sum_{i=1}^n (xi - \bar{x})^2}$$

$$= \sqrt{\frac{1}{10-1} \cdot 3566,78} = 19,90$$

$$Slog = \sqrt{\frac{1}{n-1} \sum_{i=1}^n \log^2 (xi - \bar{x})^2}$$

$$= \sqrt{\frac{1}{10-1}} \cdot 0,0760 = 0,0919$$

Variation Coefficient

$$CV = \frac{S}{\bar{X}} = \frac{19,9075}{96,4981} = 0,206$$

$$CVlog = \frac{Slog}{log\bar{X}} = \frac{0,0919}{1,976} = 0,047$$

Skewness coefficient source data

$$Cs = \frac{n \sum (xi - \bar{x})^3}{(n-1)(n-2)S^3} = \frac{107315,3223}{568044,6431} = 0,19$$

$$Cslog = \frac{n \sum \log(xi - logx)^3}{(n-1)(n-2)Slog^3} = \frac{-0,021592093}{0,055945976} = -0,39$$

Kurtosis coefficient source data

$$Ck = \frac{n^2}{(n-1)(n-2)(n-3)S^4} \sum_{i=1}^n (xi - \bar{x})^4$$

$$Ck = 3,883$$

4.5. Analysis of Design Rainfall

Calculation of design rainfall analysis is calculated using the Log Pearson type III method with the following equation.

$$\log X_{tr} = \log \bar{X} + Kt \times Slog$$

$$X_{tr} = 10^{\log X_{tr}}$$

Analysis of the design rainfall for the 25th, 50th and 100th birthday was done by calculating in tabular form as follows.

Tr	1/Tr (%)	Kt	Log Xtr	Xtr
25	4	1,699	2,088476	122,596
50	2	1,974	2,118503	131,372
100	1	2,217	2,145038	139,649

4.6 Analysis of Flood Discharge HSS-Snyder Method

Flood discharge analysis using the HSS-Snyder method was carried out at the 25, 50 and 100 year return period. The results of the flood discharge obtained are as follows.

Parameter:

Taken $Ct = 2,1$ and $Cp = 0,9$

Determine the time from the point of heavy rain to the peak discharge (tp).

$$tp = Ct \times (Lc \times L)^3$$

$$tp = 2,1 \times (48,95 \times 145,22)^3 = 30,044 \text{ jam}$$

Duration of effective rain (tr')

$$tr' = \frac{tp}{5,5} = \frac{30,044}{5,5} = 5,462$$

Because $tr' < tr$ assumption then:

$$Tp = tp + \frac{tr}{2} = 30,044 + \frac{7}{2} = 33,544 \text{ jam}$$

Determine peak hydrograph peak units (qp)

$$qp = Cp \times \frac{275}{Tp} = 0,9 \times \frac{275}{33,544} = 7,378 \text{ m}^3/\text{detik}/\text{km}^2$$

Determine peak discharge (Qp)

$$Qp = \frac{qp \times A}{1000} = \frac{7,378 \times 1158,0164}{1000} = 8,544 \text{ m}^3/\text{detik}$$

$$Qb = 0,4751 \times A^{0,6444} \times D^{0,9430}$$

$$Qb = 0,4751 \times 1158,016^{0,6444} \times 1,42571^{0,9430} = 62,55 \text{ m}^3/\text{detik}$$

Flood discharge analysis using the HSS-Snyder method was carried out at the 25, 50 and 100 year return period. The results of the flood discharge obtained are as follows.

Return Period	Flood discharge
25 Year	1104,2372 m ³ /s
50 Year	1178,8064 m ³ /s
100 Year	1249,1336 m ³ /s

4.7. Measured Discharge Calibration

Calibration is done to find out the parameters used in the flood discharge analysis are correct.

Regional Debit Analysis

Calibration is carried out using measured discharge data in the field, because measured discharge data is not available so regional discharge analysis is used to find the measured discharge from the nearest river. The river for which the debit data was collected is the Ongkag Lombagin river in 2011 carried out by:

$$Q_2 = \frac{Q_1}{A_1} \times A_2$$

with:

Q_2 = discharge of Ongkag Dumoga

A_2 = catchment area of Ongkag Dumoga

A_1 = catchment area of Ongkag Komangaan

Q_1 = measured discharge of Ongkag Komangaan

Calibration of HSS-Snyder Parameters

Calibration is done by means of the coefficient of determination test to determine the level of similarity of the model from the results of the calculated discharge and

measured discharge. Test value of the coefficient of determination > 0.6 is considered to be able to meet the

provisions for the level of similarity.

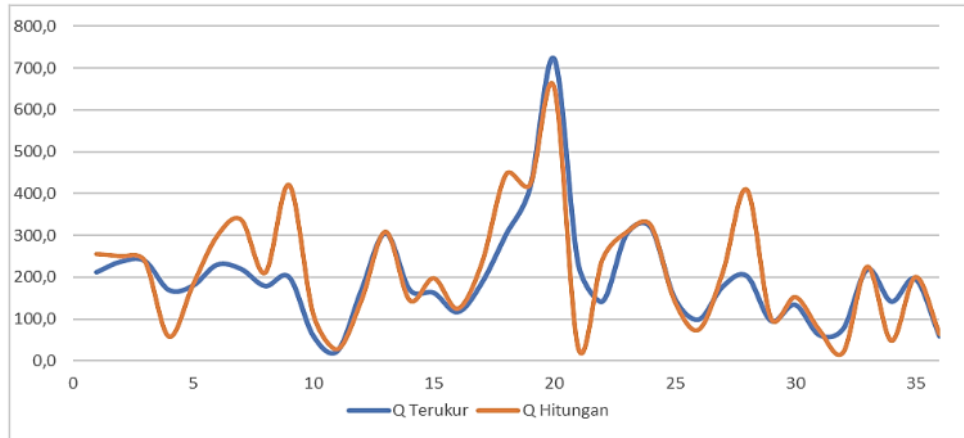


Fig 2. Discharge rating curve

The coefficient of determination test value (r^2) getting a result of 0.86 is considered to have met the similarity value of the measured discharge data, then the HSS-Snyder parameter can be used in the calculation of the planned flood discharge.

4.8. Analysis of Existing River Cross Section Hydraulics

Analysis is carried out on the existing river conditions to obtain information on water level, water surface width, channel base slope and other information on flood conditions with a specified return. The river is modeled to be crossed by the Snyder HSS flood discharge method in 25, 50, and 100 years return times using the HEC-RAS software. Simulation results can be seen in the following image.

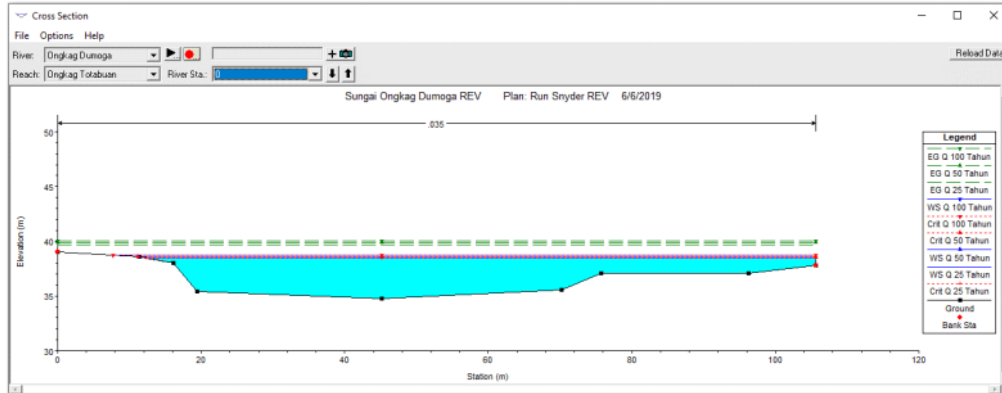
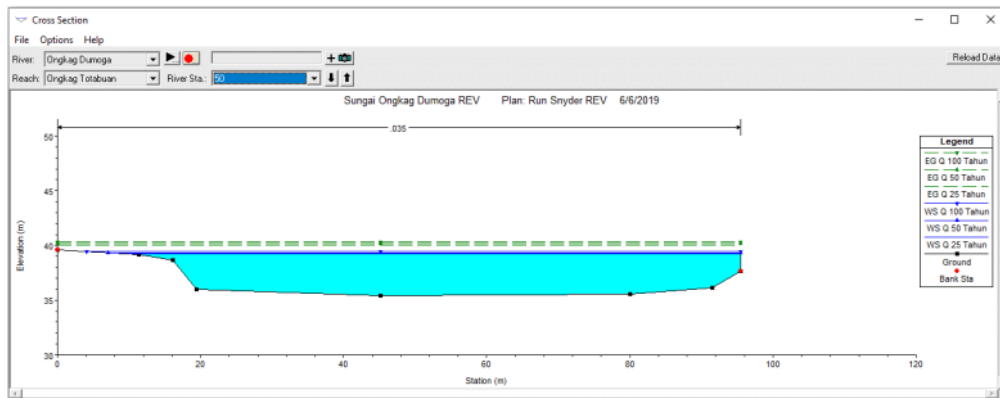
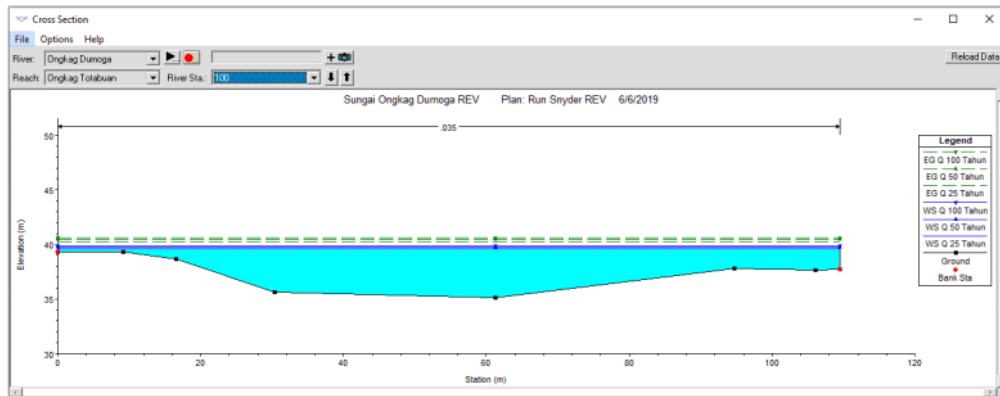
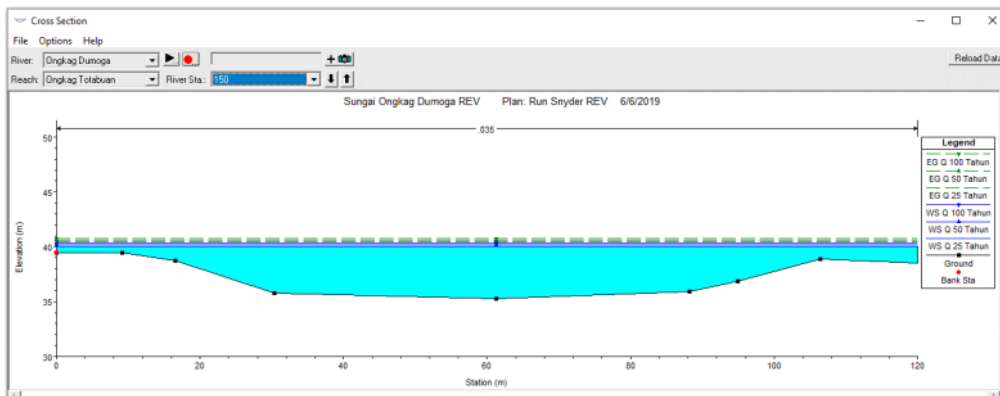
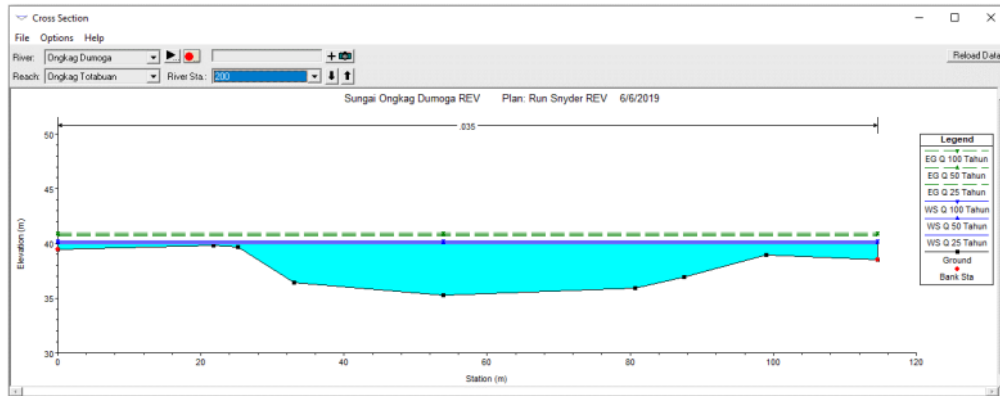
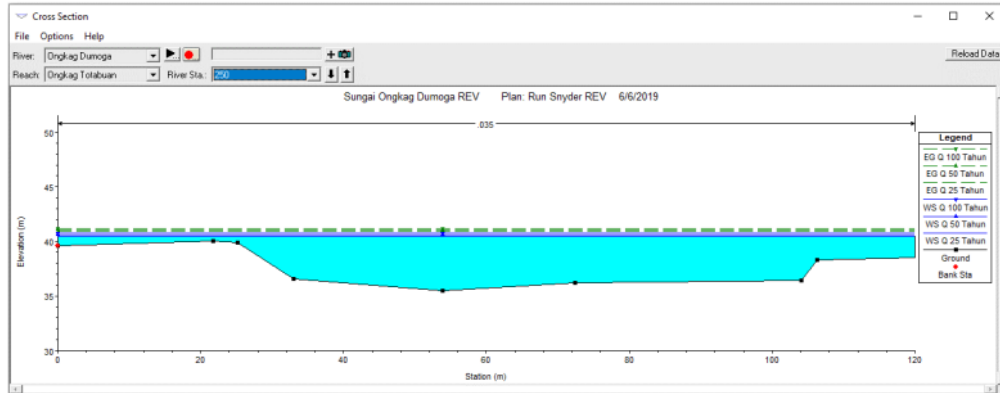
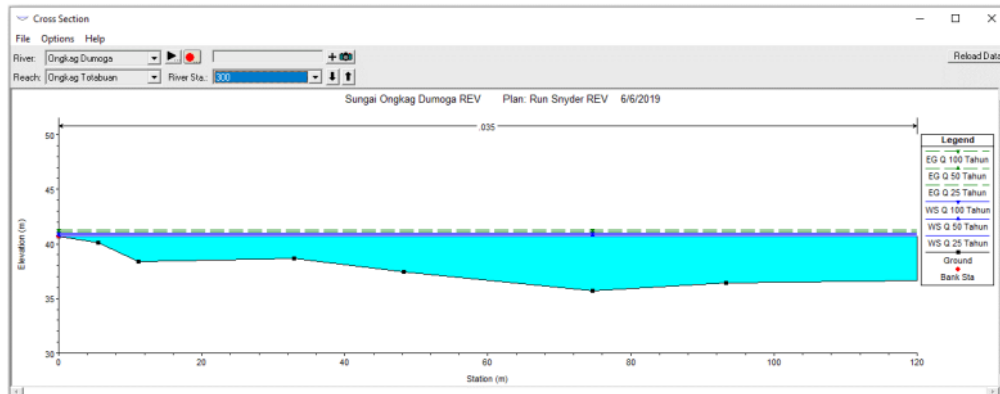


Fig 3. Water level high at STA 0

**Fig 4. Water level high at STA 50****Fig 5. Water level high at STA 100****Fig 6. Water level high at STA 150**

**Fig 7. Water level high at STA 200****Fig 8. Water level high at STA 250****Fig 9. Water level high at STA 300**

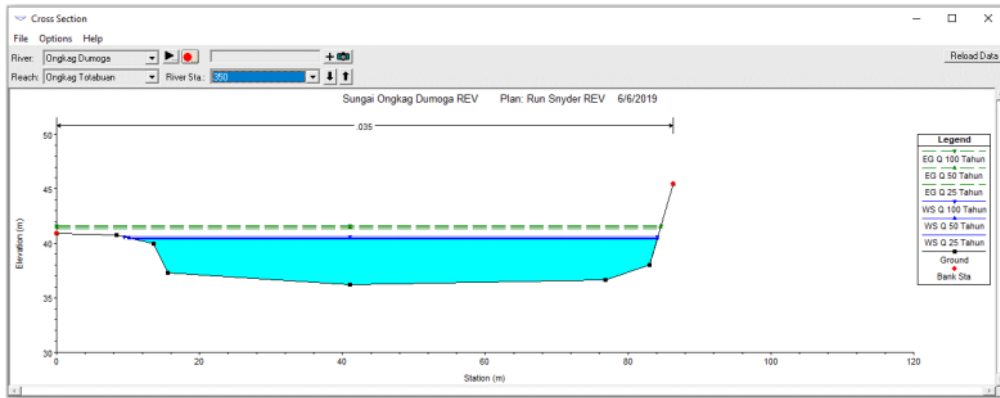


Fig 10. Water level high at STA 350

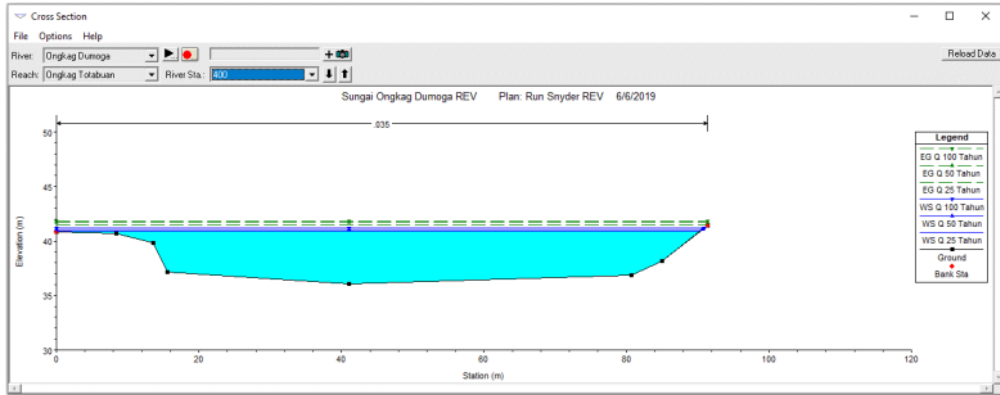


Fig 11. Water level high at STA 400

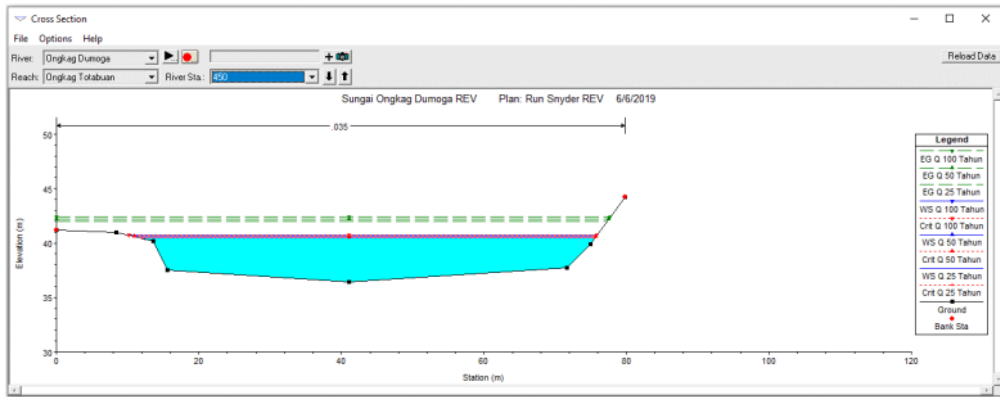


Fig 12. Water level high at STA 450

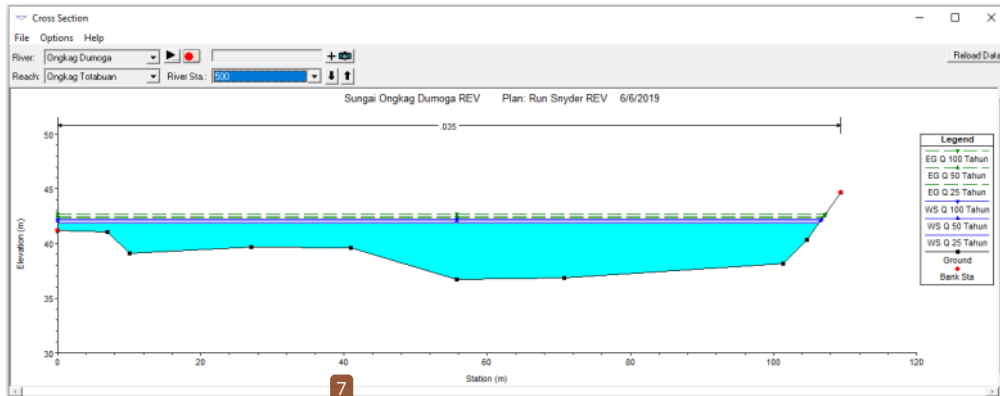


Fig 13. Water level high at STA 500

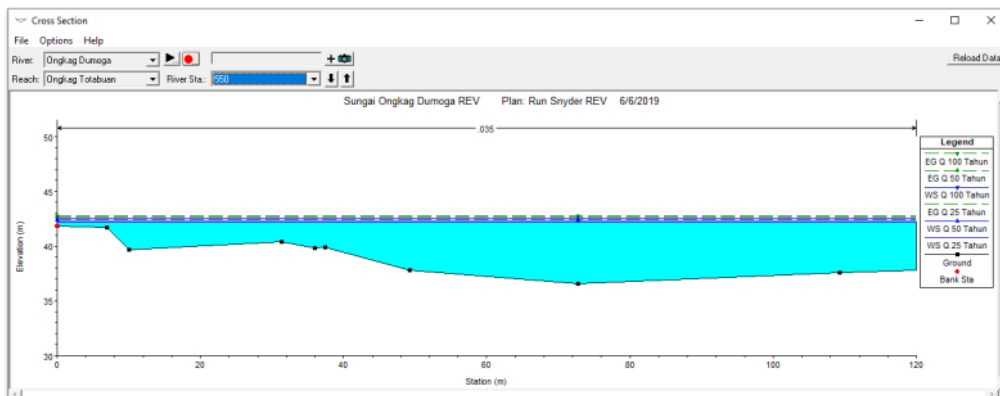


Fig 14. Water level high at STA 550

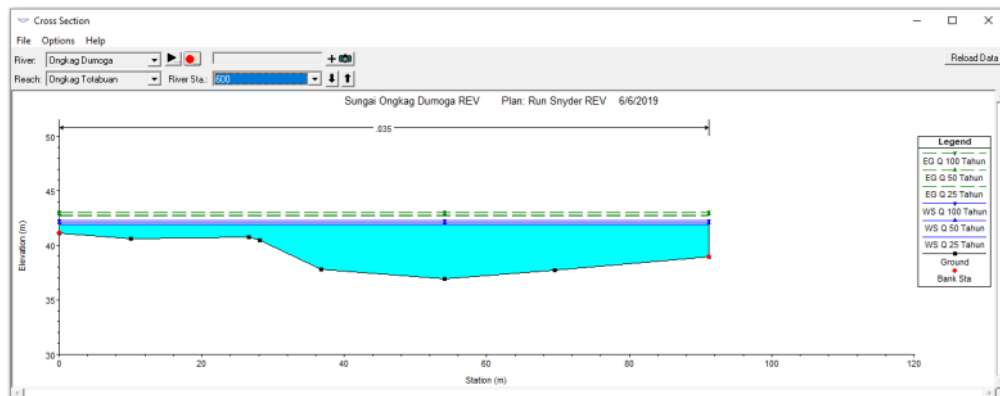


Fig 15. Water level high at STA 600

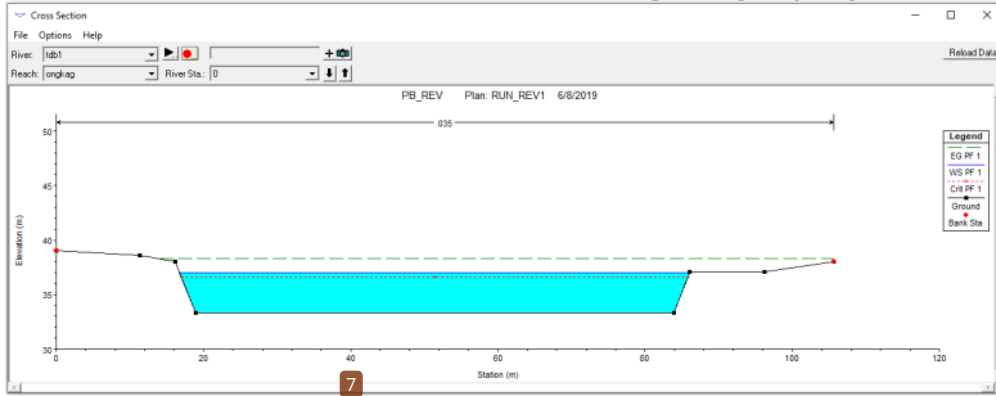
4.9. Technical Analysis of Post-Mining River Cross-Section Hydraulic

Post-mining river cross section analysis is carried out to

obtain a new river cross-section based on the economic cross section method by maintaining the slope of the original river bed. River cross sections will be analyzed to pass the HSS-

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Snyder flood discharge with a 100-year return period. The design of the new river technical appearance requires information on the average width of the existing riverbed, the slope of the riverbed, the coefficient of velocity based on river wall conditions, the depth of the potential for sand, gravel and river stone excavations and other information. This information will be included in the economical cross

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section method of the river trapezoidal cross section. Analysis of Height and Width of Surface Water of the Technical Cross Section of the Post Mining River. The Post-Mining River Technical Cross section will then be analyzed using the HEC-RAS program to find out whether the new river cross-sectional capacity has been able to accommodate flood discharges during 100 years period.



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Fig 16. Water level high at STA 0

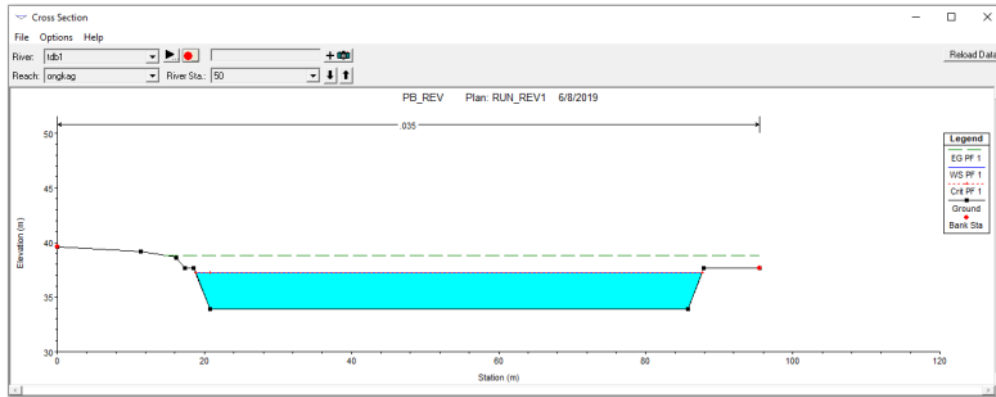
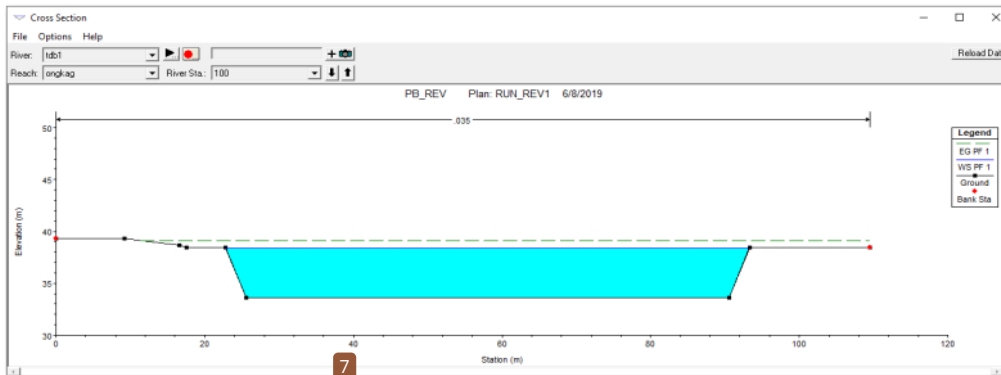
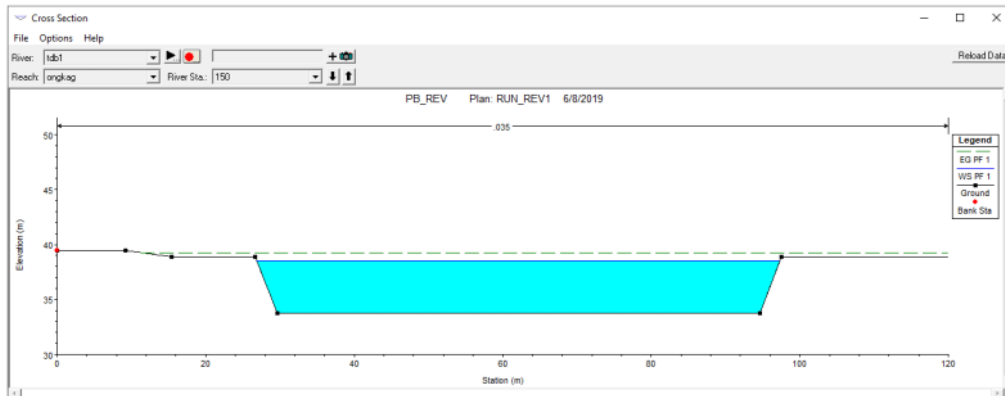
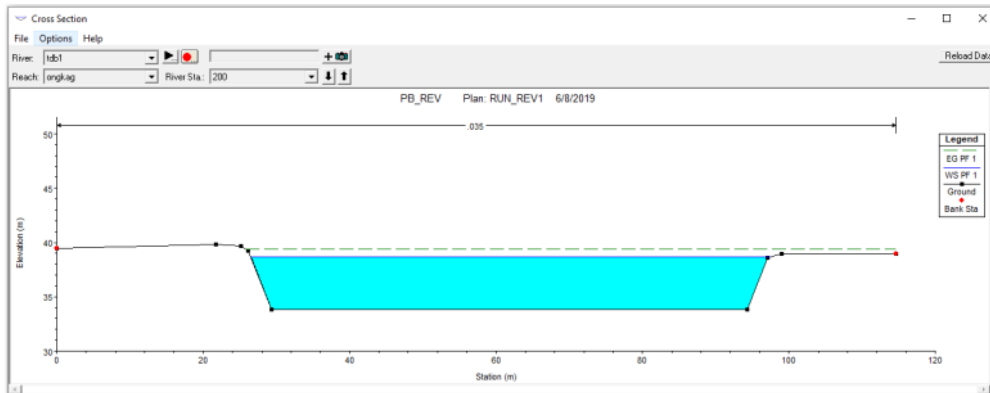
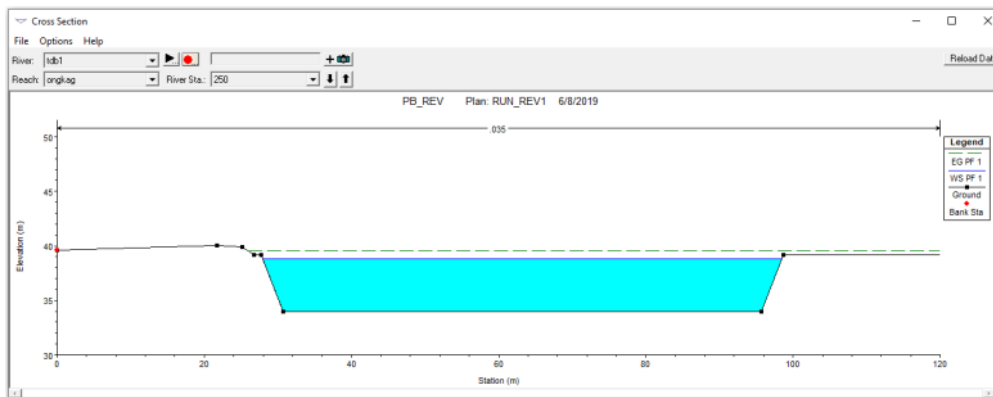
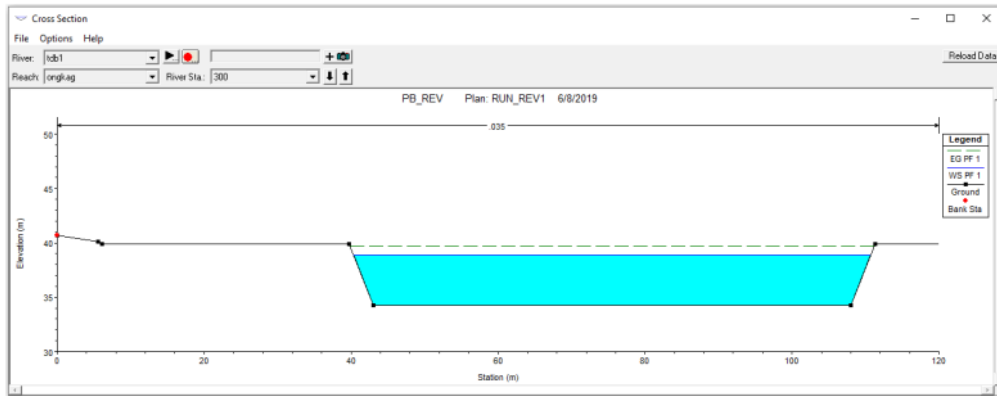
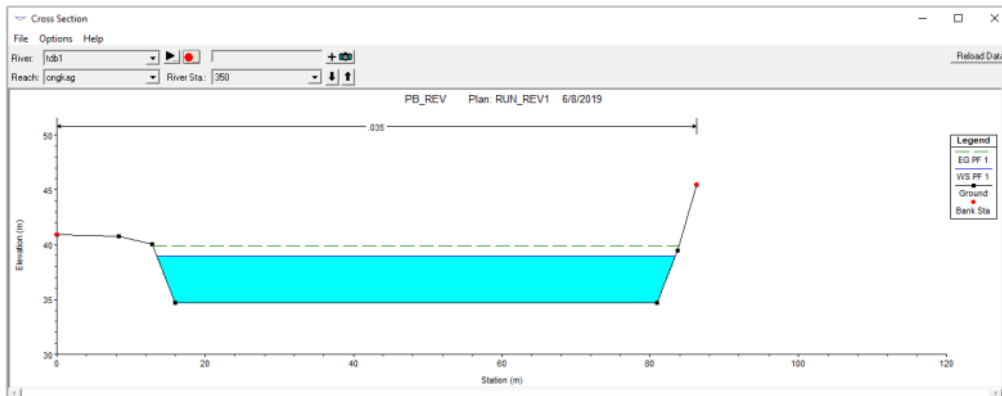
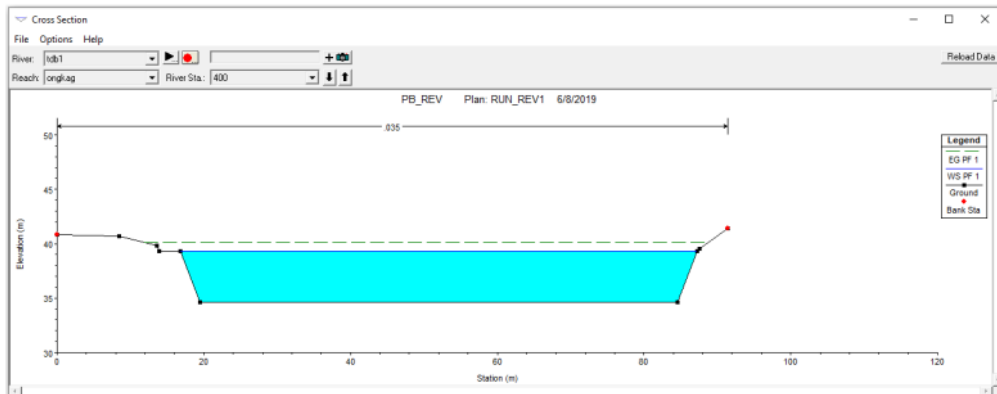


Fig 17. Water level high at STA 50



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Fig 18. Water level high at STA 100

**Fig 19. Water level high at STA 150****Fig 20. Water level high at STA 200****Fig 21. Water level high at STA 250**

**Fig 22. Water level high at STA 300****Fig 23. Water level high at STA 350****Fig 24. Water level high at STA 400**

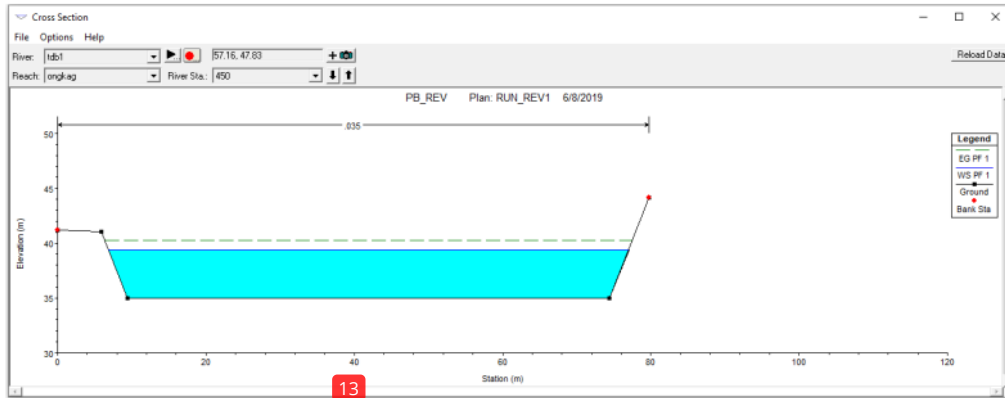


Fig 25. Water level high at STA 450

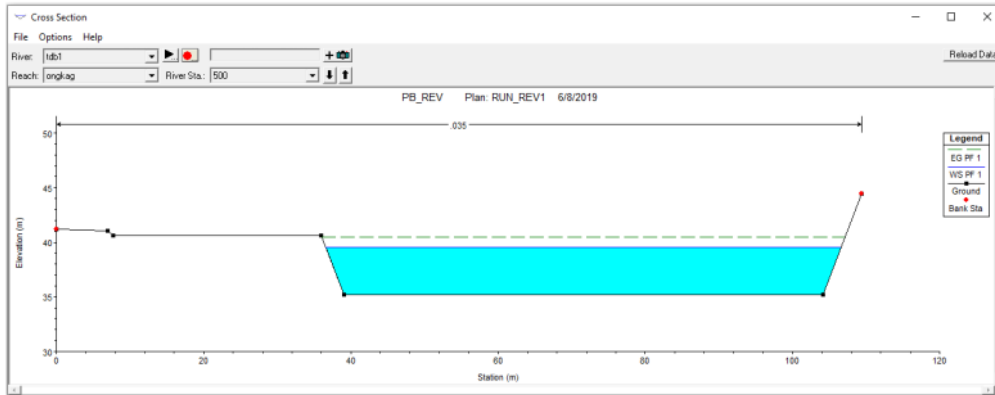


Fig 26. Water level high at STA 500

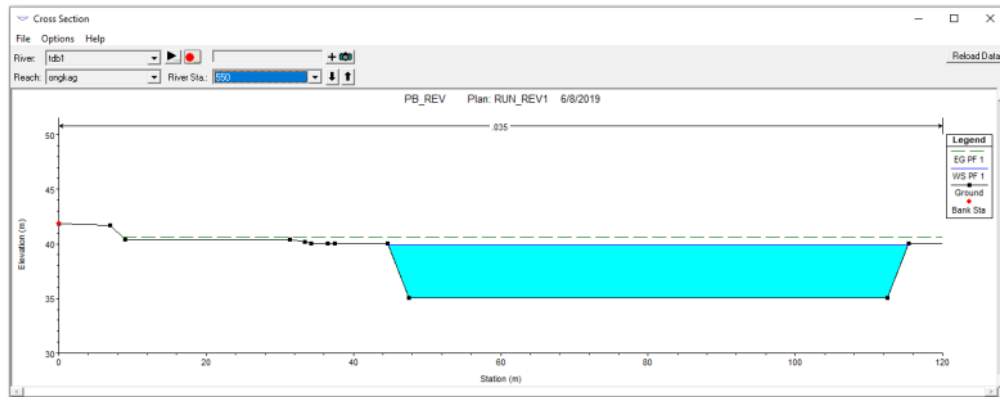


Fig 27. Water level high at STA 550

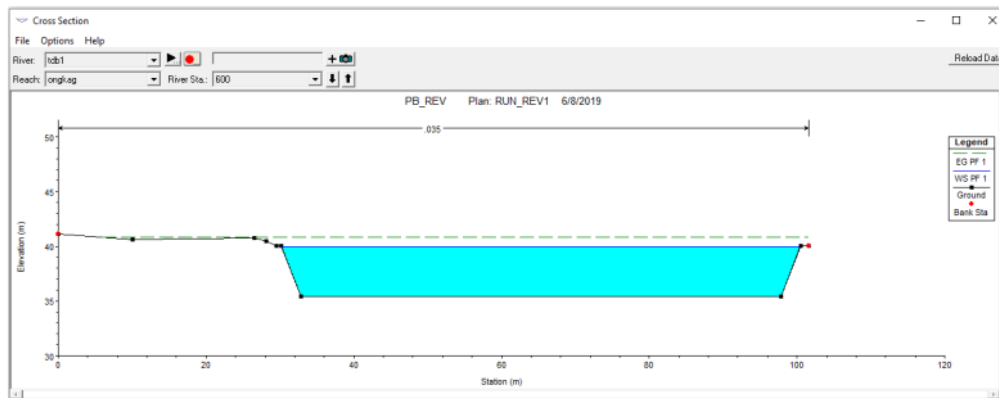


Fig 28. Water level high at STA 600

5. Result and Discussion

The height and width of the flood water level are obtained from the results of hydraulic analysis on the existing river cross section. Hydraulic analysis results show that from the simulated discharge flooding occurred in the cross section of STA 0, STA 50, STA 100, STA 150, STA 200, STA 250, STA 300, STA 400, STA 500, STA 550 and STA 600, while the overflow it does not occur only in the cross section of STA 350 and STA 450. Planning a new river profile is needed to improve the capacity of the existing river cross section.

The new river cross section design is made in a more technical river cross-sectional condition using the trapezoidal economic cross section method (Bambang Triatmodjo, 1996).

The analysis of the height and width of the flood water level results in the capacity of all technical cross sections of the new river being able to accommodate the magnitude of the simulated flood discharge. the volume of potential post-river quarrying taking into account the change in profile from the existing river to the post-mining river technical cross section. Changes in river bed elevation and water level between existing river miners and the post-mining river technical cross section can be seen.

Post Mining Material Volume Calculation, Sand and Gravel

The calculation of the potential volume of sand, gravel and river stone excavation in the Dumoga Ongkag river section is carried out in the post mining conditions. The excavation area of each segment will be calculated, then the excavation volume between two segments will be obtained by comparing the excavation area of the two segments multiplied by the length of the interval between segments.

The calculation of the potential volume of sand, gravel and river stone excavation in the Dumoga Ongkag river section is carried out in the post mining conditions. The

excavation area of each segment will be calculated, then the excavation volume between two segments will be obtained by comparing the excavation area of the two segments multiplied by the length of the interval between segments.

6. Conclusion

Analysis of flood discharge obtains Snyder HSS flood discharge which is used as a planned flood discharge.

The results of the analysis of the height and width of the flood water surface show the state of the overflowing water in several segments of the existing river at the time of return 25, 50 and 100 years.

Technical research is carried out to obtain a profile of a new river profile post mining, analysis of the height and width of the flood water level is carried out on the condition of the flood discharge 100 years of a new river profile, the results of the analysis found that the water condition has not overflowed in each new river cross section segment.

The area of new river profile changes with existing river conditions is calculated to be the potential for excavation of sand, gravel and river stone material, the quarry volume obtained in the river along the 600 m is 88,347.42 m³ while the embankment volume used to improve river cross section capacity is 13,893.85 m³.

As the suggestion, mining of sand, gravel and river stone excavation materials in order to pay attention to existing technical studies to minimize river damage that might occur to a predetermined limit.

The volume of embankment needed to improve the river cross section should use the excavated volume in the river channel.

Material scrap that can be obtained from the Dumoga Ongkag river mining must be in the form of sediment that is not native to the river

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Non Metallic Material Sand, Gravel And Stone On Ongkag River: Method of Mining

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